Understanding connected surface-water/groundwater systems using Fourier analysis of daily and sub-daily head fluctuations

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DOI:

[10.1007/s10040-014-1182-5](https://doi.org/10.1007/s10040-014-1182-5)

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Document Version

Early version, also known as pre-print

Citation for published version (Harvard):

Acworth, RI, Rau, GC, McCallum, AM, Andersen, MS & Cuthbert, MO 2015, 'Understanding connected surface-water/groundwater systems using Fourier analysis of daily and sub-daily head fluctuations', *Hydrogeology Journal*, vol. 23, no. 1, pp. 143-159. <https://doi.org/10.1007/s10040-014-1182-5>

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Journal:	<i>Hydrogeology Journal</i>
Manuscript ID:	HJ-2014-3122.R2
Category:	Paper
Date Submitted by the Author:	n/a
Complete List of Authors:	Acworth, Richard; UNSW Australia, Connected Waters Initiative Research Centre Rau, Gabriel; UNSW Australia, Connected Waters Initiative Research Centre McCallum, Andrew; UNSW Australia, Connected Waters Initiative Research Centre Andersen, Martin; UNSW Australia, Connected Waters Initiative Research Centre Cuthbert, Mark; UNSW Australia, Connected Waters Initiative Research Centre; University of Birmingham, School of Geography, Earth and Environmental Sciences
Keywords:	Australia, analytical solutions, confining units, groundwater/surface-water relations, groundwater hydraulics

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1 **Understanding connected surface-water/groundwater systems using Fourier**
2 **analysis of daily and sub-daily head fluctuations**

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13 **Abstract**

14 The long-term monitoring records of hydraulic heads frequently contain fluctuations
15 originating from different cyclic drivers. Fourier analysis applied to these records can
16 reveal connected surface-water/groundwater system characteristics. The various
17 components of the atmospheric tides, the earth tides and the presence of diurnal
18 responses to evapotranspiration are identified and isolated through band-pass filtering of
19 data recorded from both vented and absolute gauge transducers. The signature of the
20 different cyclic drivers is contained in amplitude and phase of the various signal
21 components and can be used to determine the degree of system confinement. A
22 methodology is described for the calculation of barometric efficiency in confined aquifers
23 based upon the amplitude of the M₂ and S₂ components of the earth and atmospheric
24 tides. It is demonstrated that Fourier analysis of water-level fluctuations is a simple but
25 underused tool that can help to characterise shallow groundwater systems.

26

Keywords: Australia, Analytical solutions, Confining units, Groundwater/surface-water relations, Groundwater hydraulics

1. Introduction

Signal analysis techniques are routinely used in many areas of the earth sciences (Davis, 1973) with sophisticated packages in use in the geophysics industry or the area of coastal engineering (Doodson, 1921; Emery & Thompson, 2004). Despite early work in the groundwater field by Weeks (1979), van der Kamp and Gale (1983), Hsieh et al, (1987), Rojstaczer (1988a, 1988b), Rojstaczer and Agnew (1989), Rojstaczer and Riley (1990), for example, and the regular use of signal analysis techniques in other disciplines, there is little evidence of their routine use in hydrogeological studies. This is unfortunate as this early work demonstrated that signal analysis could be used to derive a much improved understanding of the impacts of mechanical loading of confined aquifers and to the determination of barometric efficiency and specific storage. Where signal analysis techniques have been used in hydrogeology (for example Weeks (1979), Hsieh et al, (1987) or Merritt (2004)), the analysis has been for either deep confined or deep unconfined aquifer systems. Work in shallow connected surface water groundwater systems has concentrated on the diurnal variation in water level that could be related to evapotranspiration effects (Gribovszki et al 2010; Johnson et al, 2013).

The measurement of water levels for hydrogeological investigations is often considered a trivial task but needs to be undertaken carefully if the full range of important information contained in the data set is to be sensibly extracted (Post & von Asmuth, 2013). Where absolute gauge measurements (transducer diaphragm sealed on the reference side to a vacuum or a fixed pressure) are made with a transducer suspended at a fixed depth in the bore/piezometer, the logger records the total pressure on the transducer diaphragm. This includes the pressure of the water column above the point of measurement and the pressure of the overlying air. In simple terms, if the overlying air pressure can be measured by another logger separately, it can be separated from the pressure exerted by the height of the water column and removed by subtraction. This is the recommended

56 approach of many manufacturers of absolute pressure gauge transducers sold into the
57 groundwater market. The accuracy of the derived water pressure measurement is
58 therefore directly related to the accuracy of the atmospheric pressure measurement and
59 simultaneous measurements are essential. It is the necessity to accurately measure the
60 atmospheric pressure variation that has prompted our revised interest in the causes of
61 the daily and sub-daily head fluctuations caused by atmospheric pressure variation and
62 what we can learn about the aquifer system by observing this response.

63 Price (2009) has demonstrated that the data derived from the correct use of absolute
64 gauge transducers are the same as that obtained for vented gauge transducers, as long
65 as the system accuracy is sufficient. A vented gauge transducer makes the correction for
66 atmospheric pressure automatically by subjecting the opposite side of the transducer
67 diaphragm to atmospheric pressure via the use of a thin pipe extending from the
68 transducer to the atmosphere (Price, 2009). Sorensen and Butcher (2010, 2011) give an
69 extensive review of the accuracy of available logging systems.

70 In this paper we use signal analysis techniques based on the Discrete Fourier Transform
71 (DFT) on a long sequence (35,000 data points with 96 measurements per day) of
72 groundwater data from a connected surface water - groundwater environment at Maules
73 Creek in Northern New South Wales, Australia. Results are presented from a stream
74 gauge (vented transducer), loggers in two unconfined piezometers and a logger in a
75 confined piezometer (absolute gauge transducers). We describe applications of Fourier
76 signal analysis techniques to both vented and absolute gauge data and demonstrate how
77 the use of this approach can assist with hydrogeological interpretation of long data series
78 from a shallow groundwater environment connected to a stream. In particular, we show
79 how the use of a DFT pair, where the time series data is shown alongside the frequency
80 spectrum, can also be used to detect Earth tides (indicating a confined aquifer response)
81 and calculate the barometric efficiency of an aquifer, or to detect the presence of
82 evapotranspiration in a riparian zone.

2. Methodology

2.1. Catchment Description

A site on Maules Creek in Northern New South Wales, Australia (Latitude: -30.5°, Longitude: 150.08°, Elevation 253 m Australian Height Datum (AHD)), is used to demonstrate the use of the DFT pair in this study. The site has been described in previous papers (Andersen and Acworth, 2009; Rau et al., 2010; McCallum et al, 2013) and only background data will be repeated here.

Figure 1

Maules Creek is a tributary to the Namoi River that drains into the Darling River and is a part of the Murray-Darling River Basin. The creek is largely ephemeral, but has a perennial section in its middle reach at Elfin Crossing (Fig. 1) that is fed by groundwater discharge from a shallow coarse grained aquifer. At low-flow conditions, the surface-water flow in Maules Creek at this middle reach is exclusively controlled by surface-water/groundwater interactions (Andersen and Acworth, 2009), since the reaches above and below dry out except when the creek is in flood. There is a permanent flow gauge at Elfin Crossing that was established by the NSW Government with real-time data available on the web (Waterinfo, 2013). The water level at this gauging station is recorded using a vented transducer with a cable running from the pool and buried in the bank up to the gauging station hut. Details and pictures can be seen on the web site. Low flow discharge from this gauge is reported to be below approximately 10 ML/day (115 L/s).

A climate station is installed at a site at Bellevue Farm, some 11 km due west from Elfin Crossing where temperature, incoming solar radiation, wind and rainfall are measured amongst other parameters (Fig. 1).

Shallow piezometers were installed in a wooded area to the east of the creek (e.g. EC 17 on Fig. 2) using a Geoprobe pneumatic hammer to drive casing through the coarse alluvium. This method met refusal at a few metres depth. A large rotary rig equipped with a 300 mm combination percussion air-hammer and casing advance system (TUBEX) was used to achieve greater penetration on the west bank and boreholes BH 7 and BH 12

(Fig. 2) were completed, each with multiple piezometers installed isolated by a cement seal. Drill records (BH 12) indicate a sequence of sandy gravels with some clay to a depth of approximately 17 m. A clay layer is present between 25 and 30 m depth which has a significant impact on hydraulic heads with a consistent reduction in head (i.e. downward gradient) of approximately 1.25 m. Details of the piezometers and boreholes for which water level records are presented are given in Table 1.

The banks of the creek are lined with mature River Red Gums (*Eucalyptus camaldulensis*) that often have their trunks standing in surface water. The lower part of the catchment between Elfin Crossing and the junction with the Namoi River is extensively flood irrigated using a combination of groundwater abstraction from deeper parts of the alluvial aquifer and Namoi River water (Andersen and Acworth, 2009). Except under flood conditions, surface flow in Maules Creek ceases at some point between Elfin Crossing and the Namoi River as a result of losses to the underlying aquifer. The location of this cease to flow point can rapidly move upstream as a response to the start of groundwater abstraction and flow conditions at Elfin Crossing appear to be permanently impacted with significant downward gradients beneath the pool at Elfin Crossing (Rau et al., 2010) as noted above. The alluvial material that forms the base to the river channel has been cut into Permian coal measures (Maules Creek Formation) that are under active exploration by mining companies.

Figure 2

Table 1

2.2. Water level measurement

In this paper we use water levels that were recorded with a combination of vented and unvented (absolute pressure) loggers. Absolute gauge transducers (Solinst Levelogger Gold and Edge) were used at EC 17, BH 12.2 and BH 7.1 Atmospheric pressure was measured using a Solinst Barologger installed at 2 m below ground level and above the water level in BH 8 (Fig. 2). We recorded water levels with a time resolution of 96 cycles

per day (cpd) corresponding to 15 minute time intervals so that linkages between the surface stream (flood response) and the groundwater system could be accurately resolved.

2.3. The Discrete Fourier Transform (DFT)

The DFT of a long set of regularly spaced data, such as that provided by data logging of a groundwater level at a regular time interval, can be expressed in the frequency domain as a sequence of individual sinusoids (Fourier, 1822) that collectively add to make up the original signal. The mathematical expression of the DFT is as follows (e.g. Smith, 2007):

$$X(k) = \sum_{n=0}^{N-1} x(n) e^{-i2\pi \frac{kn}{N}}, \quad \text{Equation 1}$$

where: $X(k)$ is the frequency spectrum corresponding to the time series $x(n)$. In other words, Eqn. 1 transfers data from the time domain (where it is a series of measurements of a given parameter made at a constant time interval) to the frequency domain (where it can be represented by a plot of frequencies against the amplitude of that frequency). The Inverse Discrete Fourier Transform (IDFT), where data can be transferred from the frequency domain to the time domain, is defined as (e.g. Smith, 2007):

$$x(n) = \frac{1}{N} \sum_{k=0}^{N-1} X(k) e^{i2\pi \frac{kn}{N}}. \quad \text{Equation 2}$$

For a detailed discussion of the DFT and its mathematical properties the reader is referred to Oppenheim and Schaffer (1999), Emery & Thomson (2004) or Smith (2007). The graphical representation of data in both the time domain and the frequency domain can be referred to as a DFT pair (see Fig. 3 for an example).

It is noteworthy that the resolution in the frequency domain is directly linked to the resolution in the time domain, i.e. the sampling frequency of water levels. The mapping of higher frequencies is therefore limited by the sampling rate in the time domain, as the correct identification of any particular sinusoidal component requires at least 2 samples within one period. The latter is referred to as the Nyquist frequency (f_N) expressed as:

164
$$f_N = \frac{1}{\Delta t} = \frac{f_s}{z}$$
 Equation 3

165 where: Δt is the sampling time interval, or f_s is the sampling frequency. It is important to
166 acknowledge that any energy from signal components with frequencies higher than that
167 will be contained (aliased) in the spectrum but cannot be identified separately.

168 The selection of a water level sampling frequency is therefore an important consideration
169 and forms a compromise between the requirements for measuring rapid water level
170 variations and available resources for data transmission, storage and manipulations.

171 **2.4. Fourier analysis and filtering**

172 The DFT (Eqns. 1-2) is a fundamental component of many signal processing software
173 packages. It is commonly implemented as an algorithm called the Fast Fourier Transform
174 (FFT) and Inverse Fast Fourier Transform (IFFT) which efficiently solve the DFT (Eqn. 1)
175 and IDFT (Eqn. 2) numerically. Examples for popular software packages particularly
176 suited to signal processing are Matlab, Mathematica (commercial), R, Octave and PyLab
177 (open source).

178 In this paper we work with the software package TSoft (Van Camp and Vauterin, 2005).
179 TSoft is free software available for the Windows operating systems (TSoft, 2013). The
180 package allows the application of a variety of filters, including those based on the Fourier
181 Transform, and provides excellent graphical applications for data display. The results of
182 data logging can be saved as a text file and imported into TSoft using the free format
183 specifier (TSoft, 2103). It is assumed that the data is regularly sampled in time and the
184 user is prompted for a start date and time and the sample interval (s) upon data import.
185 For more details refer to the TSoft manual available online.

186 An example of Fourier analysis using the DFT on water levels is illustrated in Fig. 3 where
187 3 typical frequencies found in groundwater monitoring, viz, a signal repeating at 2 cycles
188 per day (12 hour period), 1 cycle per day (24 hour period) and at 0.2 cycles per day (5
189 day period) can be added together to show a sequence often seen in atmospheric
190 pressure data. Here, the 5 day variation represents the somewhat variable mesoscale

191 pressure variation typical for the movement of low pressure systems. This example also
192 illustrates the linearity of the DFT whereby no information is lost in the transform
193 calculation between the time domain and the frequency domain. An excellent example of
194 the use of signal processing is given by Hsieh et al (1987).

195 **Figure 3**

196 The frequency domain is particularly useful for data manipulation, as it allows the
197 extraction of a signal with a certain frequency that can be unclear in the time domain
198 data. This process is referred to as filtering. Undesired frequencies can be removed by
199 applying a weighting function to the data in the frequency domain. Three common filters
200 are available; low pass, high pass and band-pass functions. The first two only allow the
201 lower and the higher frequencies beyond a desired 'cut-off' value to pass when the signal
202 is transformed back into the time domain. The latter removes both lower and higher
203 frequencies and requires an additional bandwidth parameter defining the width of the
204 frequency window (on either side of the cut-off). Acworth and Brain (2008) illustrated the
205 use of these filters in their study of groundwater levels in shallow granites. In this paper,
206 we apply a band-pass FFT implemented in TSoft in order to investigate the phase and
207 amplitude of various frequencies at 1cpd or greater representing atmospheric tides, Earth
208 tides or other processes. The FFT uses a windowing technique to allow the variation in
209 amplitude of a specific frequency throughout the complete signal. One side effect of the
210 bandpass filtering is the edge effect introduced at the beginning and the end of the
211 filtered time series.

212 **3. RESULTS AND INTERPRETATION**

213 **3.1. Atmospheric Pressure**

214 A DFT pair of atmospheric data for a 9 month period sampled every 15 minutes (96 cpd)
215 is shown in Fig. 4.

216 **Figure 4**

217 The complete frequency spectrum of the recorded atmospheric pressure is shown with
218 data up to 48 cpd (the Nyquist frequency for logging at 96 cpd), although there is very
219 little energy for frequencies greater than 3 cpd (8 hourly). The amplitude and frequency
220 plot clearly show the higher amplitude energies associated with mesoscale pressure
221 variations at frequencies of less than 0.5 cpd. However, very clear and separate peaks
222 also occur at 1 and 2 cpd (In following figures the spectrum is truncated at 3.5 cpd and
223 amplified for clarity).

224 The peaks at 1 cpd (denoted S_1) and 2 cpd (denoted S_2) in atmospheric pressure data
225 were recognised shortly after the invention of the barometer by Torricelli in 1643
226 (Ananthakrishnan et al, 1984). There has been much debate about their origin. They are
227 not gravitational tides as they do not vary with lunar time (Thomson (Lord Kelvin), 1882).
228 It is considered that they are associated with thermal energy caused by heating the upper
229 atmosphere as the earth rotates (Palumbo, 1998). The primary generating signal is
230 approximately a square wave, corresponding to the sun rising and then setting 12 hours
231 later as the earth rotates. The amplitude and phase of this S_1 wave were extracted from
232 the signal (Fig. 4) using a FFT based band-pass filter with a cut off at 1.0 cpd and a band
233 width of 0.05 cpd. The maximum occurs at 12:00 and the minimum at 00:00. However,
234 the amplitude of this wave varies considerably throughout the seasons (Acworth and
235 Brain, 2008). By contrast, the 2 cpd wave (FFT band-pass filter with cut off at 2.0 cpd and
236 a bandwidth of 0.05 cpd) has maxima at 04:00 and 16:00 with corresponding minima at
237 10:00 and 22:00 and has almost constant amplitude with time.

238 Thermo-tidal theory (Chapman and Lindzen, 1970) is used to explain the tides with solar
239 heating and the inclusion of energy dissipation in the atmosphere due to ozone and water
240 vapour excitation being the main casual agents. In general, the S_2 is predicted by this
241 theory to have larger amplitude and is much more regular than S_1 . This is explained by
242 the S_1 tide being produced by a regionally varying number of interfering wave
243 components that are mutually destructive. It is seen that the theory accounts for most of
244 the observations but can still not account for the fact that the observed maximum in S_1
245 occurs at 10:00 local time instead of the theoretically predicted 09:00 local time.

246 There are various processes that can influence the observed water level signal in a well
247 at 1 cpd or 2 cpd. For this reason, we will refer here to the S_1 and S_2 atmospheric tides as
248 S_{1a} and S_{2a} in the analysis that follows. Irrespective of the processes responsible for the
249 formation of S_{1a} and S_{2a} we can still use this very regular excitation of the ground and the
250 response of the groundwater level, to determine barometric efficiency (Jacob, 1940). The
251 regularity of the S_{1a} and S_{2a} components is a significant advantage over using the much
252 more variable mesoscale response at frequencies below 0.5cpd and we will return to this
253 later.

254 3.2. Surface Water Levels at Elfin Crossing Stream Gauge

255 **Figure 5**

256 There is a permanent deep pool between the line of the bores and the surface water
257 transducer installation. Since a vented gauge transducer was used at this site, there is no
258 evidence of any atmospheric pressure signal in the record (Figure 5).

259 There is a small amplitude signal at 1 cpd in the amplitude and frequency plot of the
260 creek data (Figure 5). This is can be resolved using a FFT band-pass filter (cut off at 2.0
261 cpd with a bandwidth of 0.02 cpd) to be a frequency with a maximum at 08:00 in the
262 morning and a minimum at 20:00 in the evening. This should not be confused with a
263 barometric pressure response (S_{1a}) that has a maximum at 12:00 and a minimum at
264 00:00. The amplitude of the water level response is also much smaller than the S_{1a} of the
265 atmospheric data (Fig 5). Note also the lack of resolution in the output from the
266 transducer deployed at the site as the time series data for the hydraulic head shows step
267 changes.

268 3.3. Borehole Records

269 The data from BH 7.1 (Fig. 6), EC 17 (Fig. 7) on the opposite bank under the trees, and
270 BH 12.2 (Fig. 8) on the north-west bank of the area are presented. These loggers were
271 each of the absolute pressure type (Details in Table 2). To facilitate comparison, each of
272 the 3 figures shows DFT pairs of three components: the atmospheric pressure at BH 8;
273 the uncorrected output for the data logger showing the total pressure head (atmospheric

274 and water); and the hydraulic head (total pressure head with the atmospheric component
275 removed by subtraction and referenced to Australian Height Datum (AHD)).

276 **3.3.1.Borehole 7.1**

277 **Figure 6**

278 Borehole 7 contains piezometers at 248 m AHD (BH 7.1) and 242 m AHD (BH 7.2)
279 installed in a mixture of sands and gravels that are hydraulically connected to the Creek,
280 as seen from their response to floods. The middle plot in Fig. 6 shows the total pressure
281 head (water plus atmosphere) recorded by the absolute gauge pressure transducer. It is
282 clear that the total pressure head is strongly influenced by the atmospheric pressure (top
283 panel in Fig. 6) and in phase with the atmospheric pressure. The strong 1 and 2 cpd
284 spectra (S_{1a} and S_{2a} components) are clearly seen in the amplitude and frequency plot
285 shown to the right of the time series data. Note that the time series data is only a subset
286 of the complete record, selected to best represent the variability in the data set and at the
287 same time visualise the variability at the important frequencies. The Fourier analysis to
288 derive the amplitude and frequency plot was carried out on the complete record of data,
289 but in the plot, the record is shown only to 3.5 cpd as there is no significant energy
290 contained in the frequency spectrum between 3 cpd and the Nyquist frequency (48 cpd).

291 The lower plot in Fig 6 shows the hydraulic head record with the atmospheric signal
292 removed (by subtraction). The strong variability noted in the middle plot (time series) is
293 completely removed while the amplitude frequency plot shows a simple distribution with a
294 slight frequency component at 1 cpd, but no energy at 2 cpd. The S_{1a} and S_{2a} signals
295 have been eliminated.

296 In theory the water pressure at the water table represents atmospheric pressure
297 (Domenico and Schwartz, 1998; Ingebritsen et al, 2006). Accepting this definition,
298 removal of the atmospheric component from the total pressure head in a perfectly
299 unconfined aquifer should leave only the elevation head of the water table. An aquifer
300 should be considered perfectly unconfined if air can move down through the formation
301 instantaneously in response to changes in atmospheric pressure. Norum and Luthin

(1988) investigated the conditions generated by an advancing wetting front and demonstrated that confined conditions could be generated for a time if the air in the unsaturated zone was unable to escape.

As the thickness of the unconfined zone increases; resistance to air flow, the hydraulic conductivity of the material and the radius of the well can all influence the well response (Rojstaczer and Riley, 1990) producing the possibility that unconfined aquifers can show a response to atmospheric pressure change, albeit with a significant phase lag and diminished amplitude. However, the shallow well depths in this study (<30 m), the small diameter of the piezometers (50 mm) and the relatively high hydraulic conductivity of the sands and gravels all make this unlikely. The absence of any response to atmospheric pressure in the data presented (Fig 6) confirm this analysis.

Examination of the phase of the remaining 1 cpd energy using TSoft shows a maximum at 05:00 and a minimum at 17:00 and is not to be confused with the atmospheric energy at the same frequency (S_{1a}) of much greater amplitude and a different phase.

3.3.2. Piezometer EC 17

In contrast to the data from BH 7.1, EC 17 is located in trees on the opposite bank of the creek. The screened depth is approximately 6 m below ground surface with the elevation of the screen set at 4 m below the water table. The corrected hydraulic head response of this piezometer (Fig. 7) is similar to that of BH 7.1 (Fig 6) and the creek (Fig 5), confirming that this piezometer is in an unconfined portion of the aquifer system. As with the other two water table responses, there is a strong observed signal at 1 cpd that has a maximum at 06:00 and a minimum at 18:00. There is an interesting phase lag of the 1 cpd signal between BH 7.1 at 248 m AHD and EC17 at 250m AHD, of approximately 1 hour that requires further investigation.

Figure 7

3.3.3. Borehole 12.2

BH 12 was completed close to BH 7 (Figs 1 and 2) to provide a vertical profile of piezometers on the west side of the creek. BH 12.2 was completed at approximately 229 m AHD and lies beneath a confining layer of clay. It is not clear from the available drilling data how laterally extensive this clay layer is. The hydraulic head at BH 12.2 is approximately 1.25 m below that at BH 7.1, indicating a significant downward gradient.

The total pressure head is dominated by the atmospheric pressure (Fig. 8), similar to BH 7.1 and EC 17. There is also a small response at a frequency of approximately 1.9 cpd visible in the total head data (middle plot in Fig. 8). Importantly, subtraction of the atmospheric pressure leaves a residual impact where the response is now inverted (lower plot in Fig. 8) and as predicted for confined aquifers by Jacob (1940). This is clearly seen in the time series data (Fig. 9) where a reduction in atmospheric pressure is matched by an increase in hydraulic head. BH 7.1 is included in Fig.9 to demonstrate the difference in response between the unconfined BH 7.1 and BH 12.2.

Examination of the DFT pair for the corrected data (Fig. 8 lower plot) shows five small peaks that have the same frequencies as the earth tides (Bredehoeft, 1967) shown in Table 2 (Wahr, 1995; Merritt, 2004). Note also that the small response in the total pressure plot (middle plot in Fig. 8) is now recognised as the M_2 lunar frequency at 1.93 cpd.

Figure 8

Figure 9

Table 2

FFT band-pass filters were used to investigate the characteristics of the 1.0, 1.93 and 2.0 cpd signals of the BH 12.2 record in the time domain with the results shown in Fig. 10. The 1.0 cpd component has a distinct seasonality in amplitude with maxima

corresponding to the solar solstices at mid-June and mid-December with a minimum in mid-September at the equinox. This response is believed to be the result of superposition of both earth tides (K_1) and atmospheric tides (S_{1a}), noting again that S_{1a} is expected to vary seasonally. The 1.93 cpd component shows very constant amplitude throughout the record and is seasonally independent. This component is the main lunar semi-diurnal (M_2) signal emanating solely from the Earth tide response. The 2.0 cpd component shows some seasonality but less than the 1.0 cpd component and is not clearly associated with the seasons. The observed signal is believed to be the result of superposition of the S_{2a} (atmospheric tide) and S_2 (earth tide) components. The variations seen at the beginning and end of each record may be attributed to edge effects caused by the band-pass filter.

Figure 10

The various amplitude changes (Fig. 10) for the different frequency components of the hydraulic head in BH 12.2 reveal a complex situation. The observed 1.0 cpd and the observed 2.0 cpd signals in the BH 12.2 spectrum are the result of several processes. Atmospheric pressure and earth tides are both incorporated. However, the 1.93 (M_2) signal appears to only be caused by the Earth tide at this frequency. The amplitude and suggested causes of the various tide components identified by the DFT in the data are given in Table 3, with data for BH 7.1 and EC 17 included for comparison.

Table 3

4. DISCUSSION

4.1. Optimising the sampling frequency of water levels

Inspection of the atmospheric data in Fig. 4, or any of the DFT pairs (Figs 5 to 8) indicates that there is no significant information at a frequency of greater than 3 cpd. Using the Nyquist frequency, it is clear that sampling at 6 cpd will satisfactorily resolve a periodic signal at 3 cpd. Following this logic, sampling every 4 hours (6 cpd) will recover all the components of the groundwater signal. Sampling at greater than 6 cpd could therefore be considered as oversampling and wasteful of system resources. However, we acknowledge that the timing of non-sinusoidal events like the arrival of a flood peak could

382 require a higher sampling frequency depending on the desired resolution of the shape of
383 the flood hydrograph.

384 **4.2. Evapotranspiration and its spectral signature**

385 A significant 1.0 cpd signal exists in piezometers installed in the unsaturated zone
386 (Maules Creek surface water level shown in Fig. 5, BH 7.1 in Fig 6 and EC 17 shown in
387 Fig. 7). This is not an earth tide response as the other earth tide components are absent
388 and is also not a residual of an atmospheric pressure response as there is no S_{2a}
389 component. The probable explanation for this response is photosynthetic demand by
390 phreatophytes (groundwater extracting plants) on the aquifer system during daylight
391 hours (e.g. White, 1932; Loheide et al., 2005).

392 To prove this hypothesis, solar radiation is plotted together with water levels for the BH 9
393 (Fig. 2) record in Fig. 11. Although the solar radiation data is from a site 11 km distant,
394 there will not be significant variation from the Elfin Crossing site. Daily short-wave
395 radiation totals (MJm^{-2}), potential evaporation and rainfall (mm) are also shown in Fig. 11.
396 The concordance between solar radiation and potential evaporation is entirely expected.
397 The observation that water levels fall as soon as the solar radiation begins at the start of
398 the day and continues until the sun sets, after which water levels begin to recover
399 complies with the literature (e.g. Butler et al., 2007; Gribovszki et al., 2010). These data
400 show that the roots of the phreatophytes growing around the site, large river red gums,
401 reach into the gravel aquifer at Elfin Crossing and act like cyclic groundwater pumps.

402 The period between 6th and 8th November was cloudy and there was little incoming short-
403 wave radiation received. The lack of photosynthesis by the trees over this period is
404 clearly shown by the absence of the daily drawdown in the groundwater level indicating
405 that the 'groundwater pumps' had been closed down over this period.

406 **Figure 11**

407 Further detail on the photosynthetic activity can be provided by analysing the 1.0 cpd
408 signal after isolating it using a FFT band-pass filter. The time series of 1.0 cpd signals for
409 EC17, BH 7.1 and the creek are shown in Fig. 12.

Figure 12

The amplitude of the EC 17 site is considerably larger than that at BH 7.1 closer to the creek. There is also a phase lag (not shown) of between 15 min and 1 hr, with BH 7.1 leading EC 17. A very clear increase in amplitude in EC 17 and BH 7.1 is also seen as the solar radiation input increases between winter and summer.

The decrease of the fluctuation in amplitude towards the creek may be attributed to the increasing supply of surface water (Butler et al., 2007; Johnson et al., 2013) in combination with the spatial distribution of the phreatophytes (with less trees near BH 7.1). It is also interesting to note that the amplitude of the 1.0 cpd signal for the creek does not show the same degree of seasonality as that for EC 17 and BH 7.1.

Water level fluctuations induced by phreatophytes have extensively been exploited for the quantification of evapotranspiration (e.g. White, 1932; Loheide et al., 2005; Gribovszki et al., 2010) including the estimation of surface water and groundwater fractions consumed (Johnson et al., 2013). However, the spectral signature of these fluctuations and its usefulness for the determination of surface water groundwater connectivity has not yet been reported. Fig. 12 illustrates clearly the usefulness of the Fourier analysis approach.

4.3. Earth tide signals and the confined aquifer response

At the unconfined BH 7.1, air is able to move through the unsaturated zone above the water table so that the water table represents atmospheric pressure. Subtracting the atmospheric pressure signal from the total pressure, recorded by the absolute gauge transducer, completely removes the atmospheric components recorded in the total pressure record. This is clearly shown in Figs. 6 and 7 and is not surprising given the shallow depth to the water table (approx. depth 1 – 2 m), the small diameter of the piezometer tube (30 mm) and the relatively coarse grained nature of the sediments at Maules Creek. Each of these factors will act to suppress the possibility of atmospheric tides in the unconfined aquifer (Rojstaczer and Riley, 1990).

However, when the atmospheric pressure is deducted from the total pressure signal for BH 12.2, a different result occurs in that the response to atmospheric pressure is not

438 removed but (partially) inverted. This is shown in Figs. 8 and 9 and can be
439 conceptualised as the result of removing too much of the pressure signal from the record
440 thus causing the inversion. More erudite explanations are provided by Jacob (1940), van
441 de Kamp & Gale (1983), Merritt (2004), Ingebritsen et al (2006) and Price (2009): The
442 reason for the over-correction is that only a part of the atmospheric loading is initially
443 transferred to the water column in a confined aquifer with the balance of the load
444 supported by the aquifer skeleton. Subtracting all the atmospheric pressure therefore
445 provides too much of a correction. It is noted that a vented transducer will produce the
446 equivalent result with the same partial inversion of the atmospheric signal.

447 The amplitude and phase of the atmospheric pressure record at 1.0 cpd and 2.0 cpd and
448 the hydraulic head data for the BH 12.2 record are shown in more detail in Fig.13. The
449 amplitudes for these data have been kept the same in these plots so that the comparative
450 size can be appreciated. There is significant seasonal variation shown in both the 1.0 cpd
451 atmospheric data, and to a lesser extent in the 1.0 cpd data for BH 12.2 (Fig 13 a).
452 Interestingly, there is a significant phase difference between the two data sets (Fig. 13 b)
453 of approximately 5 hours. This is instead of the 12 hours that would be expected if the
454 atmospheric tide loading was expressed in the hydraulic head record as a simple
455 inversion. We take this to indicate that there is interference from an Earth tide component
456 (K_t) at the same frequency as the atmospheric pressure component (S_{1a}), but of a
457 different phase.

458 **Figure 13**

459 The 2.0 cpd signals for the atmospheric pressure and for the BH12.2 record show no
460 seasonal variation (Fig 13 c). Furthermore, the phase lag between the individual
461 components is closer (5.5 hours) to the expected 6 hours (Fig 13 d).

462 **4.4. Estimate of barometric efficiency from the ratio of the S_2 and M_2 amplitudes**

463 Jacob (1940) demonstrated that the barometric efficiency of an aquifer could be
464 calculated from the ratio of the aquifer response to the atmospheric pressure change
465 driving that response. It has been demonstrated above that the very clear atmospheric

signal at 1 and 2 cpd produces an equally clear response in the hydraulic head data at the same frequencies. However, the 1 cpd data is significantly impacted by seasonal variation and other factors so that it would be beneficial to use the 2 cpd signal for this calculation. We have also noted that the 2 cpd signal in the hydraulic head comprises the input from both the atmospheric tide (S_{2a}) and the Earth S_2 tide. It is useful to define this (hydraulic head) response as S_{2h} , where

$$S_{2h} = S_{2h-a} + S_{2h-earth} \quad \text{Equation 4}$$

We can also define a hydraulic head response to the M_2 earth tide as M_{2h} . The barometric efficiency can be calculated from the ratio of S_{2a} and S_{2h-a} .

To derive a value for S_{2h-a} , we need to find a value for $S_{2h-earth}$. Fortunately, we can use the theoretically calculated Earth tides with the measured value of M_{2h} to provide this value. The Tsoft package (Tsoft, 2013) can be used to calculate the theoretical value of the earth tides for a given latitude and time. The ratio of the amplitude of the M_2 and S_2 components of the Earth tide will be constant. We can use this ratio along with the value of M_{2h} to derive $S_{2h-earth}$.

In Fig. 14, the earth tides at 1.93 and 2.00 cpd (M_2 and S_2) are shown for Maules Creek (tide shown in red scaled on the left hand axis). The observed data for BH 12.2 over this frequency spectrum is also shown in Fig. 14 (tide shown in blue and scaled on the right-hand axis).

Figure 14

At Maules Creek, the ratio of the $S_2:M_2$ Earth tides is 0.488 (shown in Fig. 14). The M_{2h} (1.93 cpd) signal in the observed response can be multiplied by the ratio (0.488) to derive the earth tide component $S_{2h-earth}$. Subtraction of $S_{2h-earth}$ from S_{2h} allows recovery of the atmospheric component of the observed data (S_{2h-a} in Fig. 14) previously hidden in S_{2h} .

S_{2h-a} can then be used to calculate the barometric efficiency (BE) of the aquifer (Jacob, 1940; van de Kamp & Gale, 1983)

492

$$BE = \frac{dh}{dp} = \frac{s_{2h-s}}{s_{2s}} = \frac{0.000364}{0.00018} = 0.069.$$

Equation 5

493

The components in Equation 5 are given in terms of metres head, as the atmospheric

494

pressure is recorded as a head (Table 3) rather than a pressure, by the logger software.

495

The calculated value of BE is low (a rigid aquifer would have a BE of 1.0) and suggests

496

that the aquifer material is not rigid but deformable. This is entirely consistent with our

497

knowledge of the geology from the drilling records.

498

If we assume that undrained conditions apply at the frequencies involved (Rojstaczer,

499

1988) and recognise that (Jacob, 1940, van de Kamp and Gale, 1983)

500

$$BE = 1 - \gamma,$$

Equation 6

501

where γ is the loading efficiency. Parameter γ can be expressed as a ratio of terms

502

involving compressibility

503

$$\gamma = \frac{\alpha}{\theta\beta + \alpha},$$

Equation 7

504

where α is the material compressibility and β ($= 4.4 \times 10^{-10} \text{ Pa}^{-1}$) is the fluid compressibility

505

(at a temperature of 20°C), and θ is the porosity. Equation 7 can be rearranged to provide

506

solutions for α given an appropriate value for the porosity. For example $\alpha = 1.187 \times 10^{-9}$

507

Pa^{-1} for a typical porosity of 0.2. Values of compressibility for undrained and

508

unconsolidated media are not often measured as they are not of interest to the

509

geotechnical industry however Berryman (2010) provides some values for undrained and

510

unconsolidated sands that are of the same order.

511

With α , β and θ either known or assumed, the value of specific storage for the formation

512

$$S_s = \rho g (\alpha + \theta\beta)$$

Equation 8

513

can also be calculated. This approach gives a value of 1.25×10^{-5} for the specific storage

514

($\theta = 0.2$). The use of the barometric efficiency to generate a value of specific storage is

515 of great use in regional groundwater analysis (Harrington and Cook, 2011, and many
516 others).

517 5. CONCLUSIONS

518 This paper illustrates how Fourier analysis of water level data using the Discrete Fourier
519 Transform (DFT) provides a useful tool to examine water level data in the frequency
520 domain. Significant frequencies in the data can be easily resolved, in contrast to the great
521 difficulty in resolving these separate high frequency signatures in the time domain. More
522 importantly, the amplitude and phase of these frequencies can be isolated and then
523 plotted back in the time domain so that their relationship to physical processes can be
524 better explored.

525 In this paper we have made use of data sets collected with both vented and non-vented
526 (absolute gauge) transducers. Inspection of this data in the frequency domain reveals the
527 presence of significant tides at a frequency equal to, or greater than, one cycle per day.
528 These are generated by a mixture of thermally derived atmospheric tides, earth tides and
529 changes caused by variation in evapotranspirative fluxes. The data for a variety of
530 unconfined and confined head data is analysed.

531 Unconfined aquifer data shows that the thermally induced atmospheric tides are not
532 retarded in their progression through the unsaturated zone and they are completely
533 removed by subtraction of the atmospheric pressure from the total pressure measured by
534 absolute gauge transducers installed at the site.

535 The recognition of the characteristic Earth tide frequencies (O_1 , K_1 , M_2 , S_2 and N_2) in the
536 frequency spectrum for a deeper piezometer at the site indicates that the aquifer is
537 confined at this location. Under confined conditions, the response at the piezometer is
538 formed by a mixture of thermal atmospheric tides and earth tide components. A method
539 of separating these components is described and the barometric efficiency is determined.

540 Fourier analysis also helps to determine the best sampling frequency for long-term
541 groundwater monitoring by considering the necessary resolution in both time and
542 frequency domain. The analysis demonstrates that a sampling interval of 4 hours (6 cpd)

543 is sufficient to capture the essential system characteristics illustrated in this study. It is
544 recommended that data logging for long-term groundwater monitoring move towards the
545 less frequent measurement unless there are other grounds for maintaining more frequent
546 measurements.

547

548 **Acknowledgements**

549 Aspects of this paper were first presented in September 2012 at the IAH 49th Congress in
550 Niagara Falls, Canada. The site at Elfin Crossing on Maules Creek has been established
551 using funds provided by the Cotton Catchment Communities CRC as a part of their
552 Catchment Research program. Funding for GCR was provided by the National Centre for
553 Groundwater Research and Training, an Australian Government initiative, supported by
554 the Australian Research Council and the National Water Commission. The NSW Office of
555 Water maintains the Elfin Crossing Stream gauge. Borehole and logger installations were
556 only possible by funding from The Australian Government Groundwater Educational
557 Investment Fund (GEIF). MOC was supported by the European Community's Seventh
558 Framework Programme [FP7/2007-2013] under grant agreement n°299091. The authors
559 are grateful to Edwin Weeks and two other anonymous reviewers for their helpful
560 comments.

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697 in red and scaled on the left-hand axis. The measured spectrum of the BH 12.2
698 response is shown in blue and scaled on the right-hand axis.
- 699

700 15.

701 Table 1. Construction and logger details for the piezometers used in this study (locations
702 are shown in Fig. 2). All unvented loggers were manufactured by Solinst for a range of 20
703 m (1.5 m for the baro logger).

Name	Elevation to top of casing (m AHD)	Screen length (m)	Elevation of mid- point of screen (m AHD)	Logger ID
BH 7.1	258.721	0.15	248.026	2004775 Edge
BH 7.2	258.721	1.5	242.241	1058991 Gold
BH 8 (baro logger)	258.382	1.5	253.587	1044805 Gold
BH 9	254.275	1.0	241.640	1044960 Gold
BH 12.2	259.164	0.15	229.389	2003344 Edge
EC17	255.67	0.15	250.180	1057878 Gold

704

705 Table 2. Principal solar and lunar earth tide components.

Symbol	Frequency (cpd)	Period (hours)	Vertical amplitude (mm)	Explanation
O ₁	0.92953	25.819	158.11	Main lunar diurnal
K ₁	1.00273	23.934	191.78	Lunar-solar diurnal
M ₂	1.93227	12.421	384.83	Main lunar semi-diurnal
S ₂	2.00000	12.000	179.05	Main solar semidiurnal
N ₂	1.89598	12.658	73.69	Lunar elliptic (lunar semi-diurnal)

706

707

708
709 Table 3. Amplitude and probable cause of the observed spectra at 1.00, 1.93 and 2.00
710 cpd frequencies.

Location	Tide component (cpd)	Amplitude (mm)	Probable Cause
Bore 8 (baro logger)	1.0	7.10	S _{1a}
	2.0	8.15	S _{2a}
BH 7.1	1.0	0.57	Evapotranspiration
BH 12.2	0.93	0.55	Earth tide O ₁
	1.0	0.68	S _{1a} mixed with earth tide K ₁
	1.90	0.26	Earth tide N ₂
	1.93	1.16	Earth tide M ₂
	2.0	1.13	S _{2a} mixed with earth tide S ₂
EC17	1.0	1.15	Evapotranspiration
	2.0	0.3	Evapotranspiration
Stream	1.0	0.55	Evapotranspiration

711

1. Location of the Maules Creek Catchment in New South Wales, Australia.
204x261mm (300 x 300 DPI)

2. Cross section for Elfin Crossing boreholes.
87x29mm (300 x 300 DPI)

3. Illustration of the Discrete Fourier Transform (DFT) using simple sine waves: a) 1 cycle per day (cpd) and b) the DFT of a; c) 2 cpd and, d) the DFT of c; e) 0.2 cpd, and f) the DFT of e; g) the summation of these three components (1 cpd, 2 cpd and 0.2 cpd), and h) the resulting DFT.
149x111mm (300 x 300 DPI)

4. The atmospheric record for the Barologger in BH 8 showing the DFT pairs: a) pressure and temperature, and b) the DFT of the atmospheric pressure. The temperature record from the barlogger (red line) is included in part a) to demonstrate that there is no temperature dependence on the pressure record. 165x102mm (300 x 300 DPI)

5. DFT pairs for atmospheric pressure observed at BH 8 and the Elfin Crossing surface water level gauge. The complete spectrum is shown in the frequency amplitude plot. a) Time series of barometric pressure; b) DFT of a; c) time series of Elfin Crossing, and d) DFT of c.
130x64mm (300 x 300 DPI)

6. DFT pairs for piezometer BH 7.1, installed at 10 m depth on the north-west bank of Elfin Crossing (see Fig. 2 for location); a) Time series of barometric pressure and b) is the DFT of a; c) time series of BH 7.1 total pressure and d) is the DFT of c; e) the time series of BH7.1 water levels and f) is the DFT of e. 188x132mm (300 x 300 DPI)

7. DFT pairs for piezometer EC 17, installed at 5.5 m depth on the opposite bank beneath tree cover (see Fig. 2 for location); a) Time series of barometric pressure and b) is the DFT of a; c) time series of EC 17 total pressure and d) is the DFT of c; e) the time series of EC 17 water levels and f) is the DFT of e. 188x130mm (300 x 300 DPI)

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187x129mm (300 x 300 DPI)

9. Comparison of atmospheric pressure and corrected hydraulic heads for BH 7.1 and BH 12.2. Note the inverted phase of the atmospheric pressure signal in BH 12.2 data and also the completely different phase and frequencies of the unconfined signal of BH 7.1.

180x119mm (300 x 300 DPI)

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243x331mm (300 x 300 DPI)

11. Composite data for BH 9 at installed at 12.7 m depth (close to EC 17) showing water level change, solar radiation and potential evaporation.
175x111mm (300 x 300 DPI)

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266x369mm (300 x 300 DPI)

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